

BUILDING SPECIFICATIONS

The manufacturer is not responsible for the concrete foundation design. The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction materials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The manufacturer will assume no responsibility for any loads not indicated.

This manufactured building is designed with the manufacturer's standard design practices which are based on pertinent procedures and recommendations of the following organizations and codes :

- American Institute of Steel Construction "Specification for the design fabrication and erection of structural steel for buildings" 13th edition.
- American Iron and Steel Institute "Specification for the design of cold formed steel structural members" 2001 edition.
- American Welding Society "Structural Welding Code" AWS D1.1
- Metal Building Manufacturers Association "Specification for the design fabrication and erection of the structural system" most current edition.

Material properties of steel plate and sheet used in fabrication of primary rigid frames and all primary structural framing members (other than cold-formed sections) conform to ASTM A-529 or A-572 all with a minimum yield point of 55 KSI.

Material properties of cold formed light gage steel members conform to the requirements of ASTM A-653, with a minimum yield point of 55 KSI

High strength bolts and their installation shall conform to ASTM specification A-325 and are designed as bearing type connections with threads included in the shear plane. Tightening of these bolts is recommended by the Turn-of-the-nut method, per "AISC Specification for Structural Joints."

All primary structural members except bolts and fasteners shall receive one coat of Iron Oxide inhibitive primer.

Shop and field inspections and associated fees are the responsibility of the contractor, unless stipulated otherwise.

CONTRACTOR RESPONSIBILITIES

The contractor must secure all required approvals and permits from the appropriate agency as required.

Approval of the manufacturer's drawings and calculations indicate that the manufacturer has correctly interpreted and applied the requirements of the contract drawings and specifications. (AISC 303-05 Code of Standard Practice)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern. (Section 3.3 AISC 303-05 Code of Standard Practice)

Design considerations of any materials in the structure which are not furnished by the manufacturer, are the responsibility of the contractor and engineers other than the manufacturer's engineering, unless specifically indicated.

The contractor is responsible for all erection of steel and associated work in compliance with the manufacturer's "For Construction" drawings.

Temporary supports, such as guys, braces, flashwork or other elements required for the erection will be determined and furnished and installed by the erector. (Section 7 AISC 303-05 Code of Standard Practice)

It is the contractors responsibility to apply or observe all pertinent safety rules and regulations, as per OSHA standards as applicable.

The Contractor is responsible for the verification of all shipments received. Any "external" damage or shortages must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim. NO EXCEPTIONS!!!

POWERBILT STEEL BUILDINGS



DESIGN LOADING

This structure is designed utilizing the loads indicated and applied by the :
IBC 2006

It is the contractor's responsibility to confirm that these loads comply with the requirements of the local building department.

Specific loads : (See structural calculations and foundation reactions.)

- 20 PSF Live Load
- LL Reduction Allowed : Yes No
- 15 PSF Ground Snow Load
- 1.2 Thermal Factor (Ct)
- 1.0 Snow Exposure Factor (Ce)
- 90 MPH Wind Load Exposure B (If applicable)
- 2.2 PSF Dead Load (Metal Bldg. Weight - Purlins, Panels, Etc.)
- 1 PSF Collateral Load (Lighting)
- 2 Use Category (I_v= 1.00 I_s= 1.00 I_e= 1.00)

DRAWING INDEX

- CS-1 Drawings Cover Sheet
- CS-2 Fastener Schedule
- E1 Anchor Bolt Plan
- E2 Anchor Bolt Details & Notes
- E3 Rigid Frame Elevation
- E4 Rigid Frame Elevation
- E5 Sidewall Framing
- E6 Endwall Framing
- E7 Roof Framing & Sheeting
- E8 Sidewall Sheeting
- E9 Endwall Sheeting
- E10 Detail Drawings
- E11 Trim Drawings

SEISMIC DATA :

- 1) Mapped Spectral Acceleration for Short Period, S_s 3.369
- 2) Site Coefficient, F_a 1.00
- 3) Seismic Design Category = D
- 4) Site Class = D
- 5) Basic Structural System and Seismic Resisting System
Ordinary Moment Frame of Steel
- 6) Frames: R = 3.25
- 7) Cables: R = 3.25
- 8) Analysis Procedure = Equivalent Lateral Force

These Drawings are for :

- Construction Approval *
- Permit Anchor Bolts & Reactions

* Approval orders must be released for fabrication within thirty (30) calendar days after the submittal drawings are issued or they will be subject to any current price increases. Special attention should be given in approving dimensions and/or details. Please verify requested dimensions by indicating 'OK'.

Building is manufactured by STEEL BUILT CORPORATION. STEEL BUILT CORPORATION is a fabricator approved by the following agencies.

1. QUASAR/CWB GROUP - CAN/CSA A660 Certificate # STEBUO
2. INTERNATIONAL ACCREDITATION SERVICE, INC.
Fabricator Inspection Program FA - 446
3. City of Houston Approved Fabricator Registration Number - 759
4. City of LA Approved Fabricator
Type I FABRICATOR LICENSE NUMBER - #2091

Engineering Seal

This certification covers parts manufactured and delivered by the manufacturer only, and excludes parts such as doors, windows, foundation design and erection of the building.

DSN: SJ	DWN: JMC	REV:
DET:	CHK:	REVISIONS
		NO.
		DATE
SCALE:		DATE:
NOT TO SCALE		12/23/10
		JOB NO:
		15017
		SHT. NO:
		CS-1

DRAWINGS COVER SHEET

CUSTOMER :
MISSOURI D.O.T.
CHARLESTON, MO
MISSISSIPPI COUNTY

POWERBILT STEEL BUILDINGS
1559 LASKIN ROAD
VIRGINIA BEACH, VA

NOTICE

Be advised that all shipments are via outside trucking companies and are signed for in good condition and completeness at the pick-up location by the driver.

"No claim will be honored" unless the following procedures have been followed:

AT TIME OF RECEIPT: At the delivery point the driver is required to provide you with a bill of lading which lists and illustrates all bulk items to be received. Any "external" damage or shortage must be noted on all copies of the bill of lading and one copy is to be retained for your records. Failure to do so will make it impossible for the factory to honor any claim.
NO EXCEPTIONS!!!

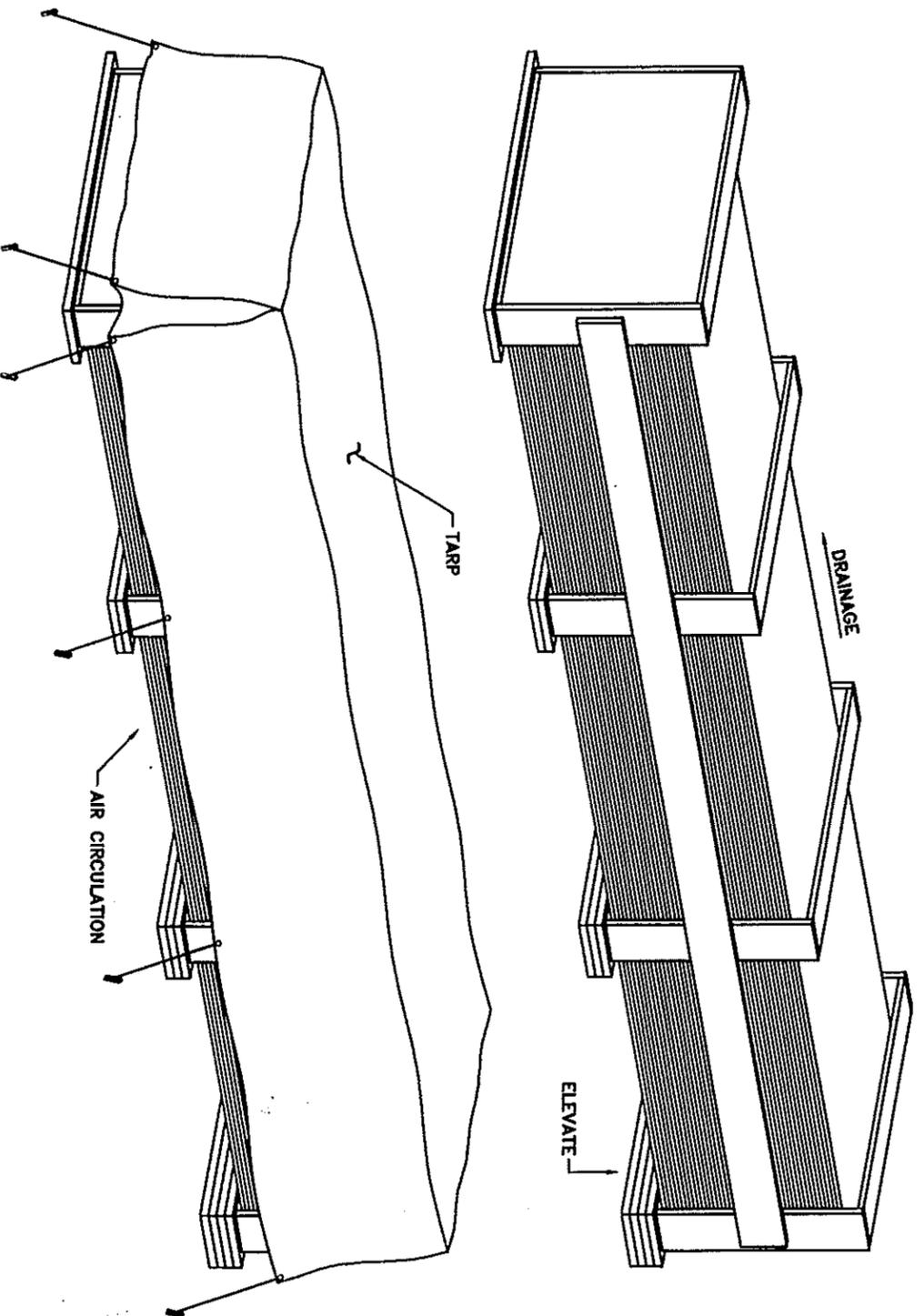
CUSTOMERS DETAILED It is a factory required policy that the customer receives a detailed shipping list (packing list) that specifies all included quantities of individual parts. Valid concealed shortages must be reported to the factory within Valid 30 calendar days **NO EXCEPTIONS!!!** The driver does not have to be present for your inventory confirmation.

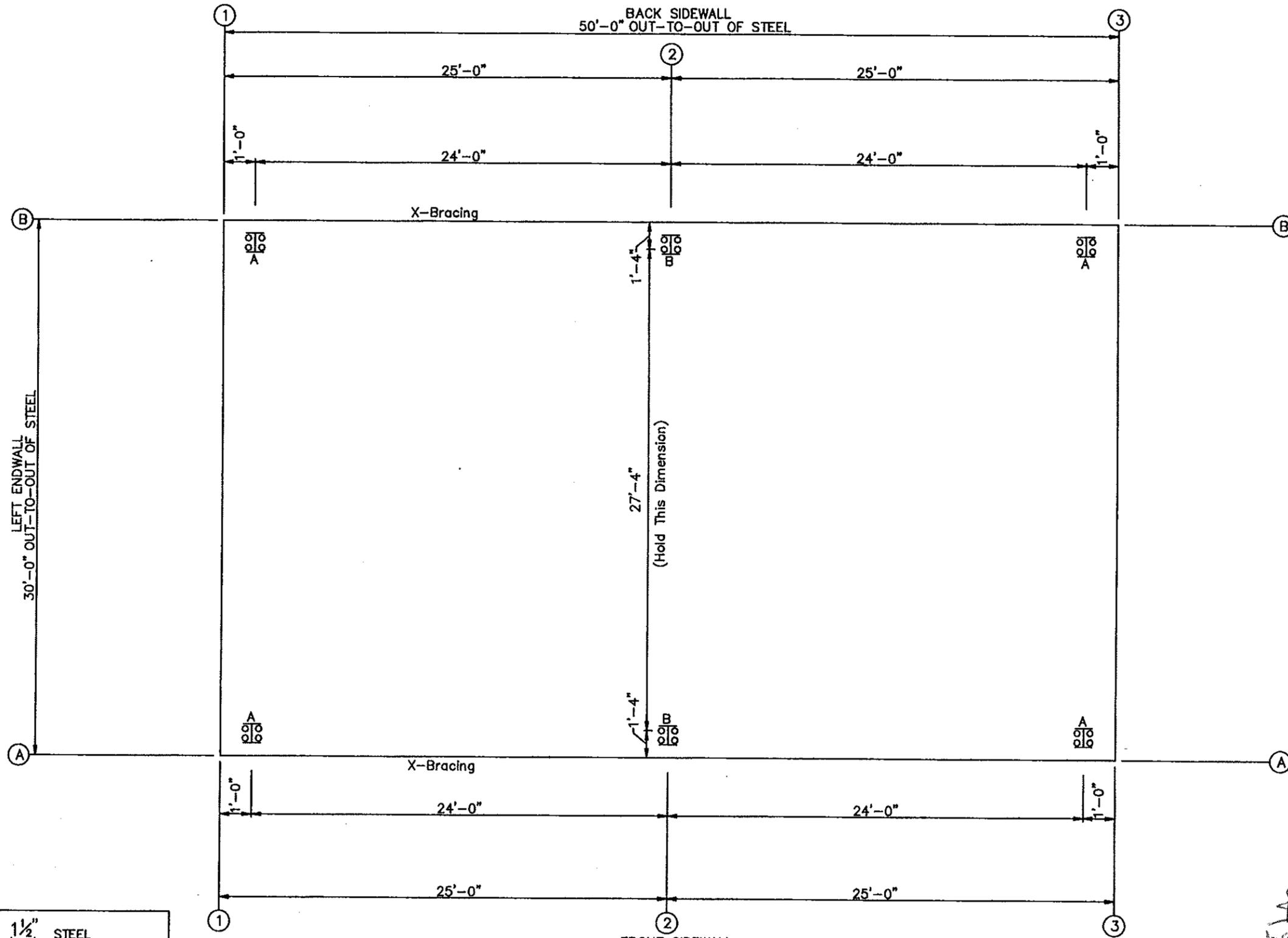
SPECIAL NOTE #1: Overhead doors, windows and service doors are shipped in cardboard boxes. It is required that they be opened to insure that the trucker's chains have not damaged the edges. If so, it **must** be noted on the bill of lading.

SPECIAL NOTE #2: PERMIT DRAWINGS are **NOT** to be used for construction purposes. Dimensions and details on permit drawings may vary with construction drawings. Use construction drawings **ONLY** for the erection of your building.

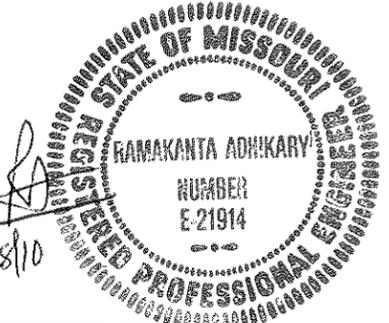
NOTICE

If water is allowed to remain on or in bundles of painted or coated parts for extended periods of time degradation will occur. The end result is shorter material life due to corrosion. Therefore, upon receipt of a job, all bundles of primed parts should be stored at an angle to allow any trapped water to drain away and permit air circulation for drying. Separate the panels to allow air to circulate between each panel. If environmental conditions are such, tarps should be used to protect materials. Puddles of water should not be allowed to collect and remain on columns or rafters for the same reason.





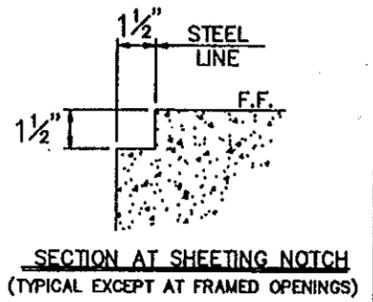
ANCHOR BOLT PLAN
 NOTE: All Base Plates ⌀ 100'-0" (U.N.)



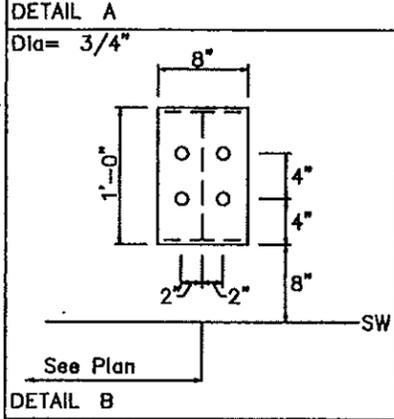
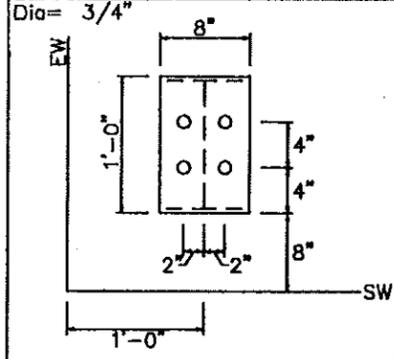
BASE NOTCH DETAIL
 REQUIRED WHEN BASE TRIM IS NOT ORDERED.

BASE FOUNDATION NOTCH WILL REQUIRE THE PERIMETER OF THE FOUNDATION TO BE 1 1/2" WIDER THAN SHOWN.

(SEE DETAIL TO THE RIGHT.)



POWERBILT STEEL BUILDINGS		Customer: MISSOURI D.O.T.	
City: VIRGINIA BEACH	State: VA	City: CHARLESTON	State: MO
Designer: SJ	Date: 12/22/10	Drafter: JMC	Date: 12/23/10
Detailer:	Date:	Office:	Job No.: 15017
Checker:	Date:	Office:	
ANCHOR BOLT PLAN			Sht. E1 of 11



NOTES FOR REACTIONS

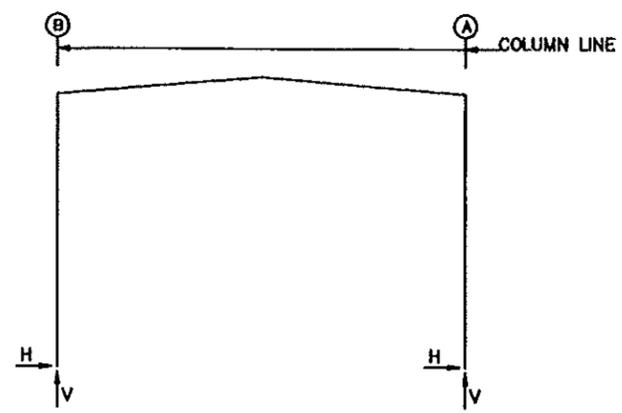
Building reactions are based on the following building data:

Width (ft)	=	30
Length (ft)	=	50
Eave Height (ft)	=	20 / 20
Roof Slope	=	1.0:12 / 1.0:12
Dead Load (psf)	=	2.200
Collateral Load (psf)	=	1
Roof Live Load (psf)	=	20.00
Roof Snow Load (psf)	=	15
Wind Speed (mph)	=	90
Wind Code	=	IBC 06
Wind Exposure	=	B
Closed/Open	=	Partial
Importance - Wind	=	1.00
Importance - Seismic	=	1.00
Seismic Design Category	=	D
Seismic Coeff (Fa*Sa)	=	3.370

Load Combinations

DL+CL+(LL or SL)
DL+CL+W
DL+CL+0.75WL+0.75(LL or SL)
DL+0.75(0.7SEIS)+0.75(LL or SL)
0.8DL+W
0.8DL+0.7SEIS

FRAME LINES: 1 2 3



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt No	Bolt D(in)	Base Plate Wid	Base Plate Len	Base Plate Thk	Grout (in)
1	B	4	0.750	8.000	12.00	0.500	0.0
1	A	4	0.750	8.000	12.00	0.500	0.0

1 * Frame lines: 1 3

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt No	Bolt D(in)	Base Plate Wid	Base Plate Len	Base Plate Thk	Grout (in)
2	B	4	0.750	8.000	12.00	0.500	0.0
2	A	4	0.750	8.000	12.00	0.500	0.0

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF THE BUILDING MANUFACTURER.
- THE BUILDING REACTION DATA, REPORTS THE LOADS WHICH THIS BUILDING PLACES ON THE FOUNDATION.
- COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 2500 POUNDS PER SQUARE INCH.
- ALL ANCHOR BOLTS SHALL BE ASTM A307 OR EQUAL IN ORDER TO CONFORM TO THE BUILDING MANUFACTURER, DESIGN ASSUMPTIONS BASED ON THE ALLOWABLE STRESSES GIVEN IN THE AISC MANUAL OF STEEL CONSTRUCTION.
- ANCHOR BOLTS TO BE SUPPLIED BY OTHERS, ANCHOR BOLT EMBEDMENT LENGTH IN CONCRETE TO BE DESIGNED BY OTHERS.
- ANCHOR BOLT PROJECTION ABOVE CONCRETE FINISHED SURFACE TO BE 3" UNLESS OTHERWISE NOTED BY FOUNDATION DESIGNER.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLERANCE OF +/- 1/8" IN ELEVATION AND LOCATION.
- IF THIS PROJECT HAS BYPASS GIRTS, IT WILL REQUIRE THE SIDEWALL COLUMN BASES TO BE SET BACK FROM SIDEWALL STEEL LINE BY THE DEPTH OF THE GIRT SYSTEM (8" or 10"). THIS MAY REQUIRE THE SIDEWALL FOOTINGS TO INCREASE IN DEPTH.
- MINOR FIELD WORK OF STRUCTURAL SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Snow---		---Wind_L1---		---Wind_R1---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	B	0.2	1.5	0.1	0.5	0.9	5.6	1.2	7.0	-2.2	-9.1	4.4	-3.4
2	A	-0.2	1.5	-0.1	0.5	-0.9	5.6	-1.2	7.0	-4.4	-3.4	2.2	-9.1
2	B	-5.9	-2.7	0.7	3.0	2.2	-8.4	2.5	-6.5	-2.8	3.8	2.8	3.8
2	A	-0.7	3.0	5.9	-2.7	-2.2	-8.4	-2.5	-6.5	-2.8	3.8	2.8	-3.8
2	B	0.0	-7.8	0.0	-0.8	-0.1	-0.1	0.0	-0.3	0.0	0.0	0.0	0.0
2	A	0.0	-7.8	0.1	-0.1	0.0	-0.8	0.0	0.0	0.0	-0.3	0.0	-0.3
1	B	0.1	0.9	0.0	0.2	0.5	3.2	0.5	2.9	-1.1	-4.1	1.6	-1.6
1	A	-0.1	0.9	0.0	0.2	-0.5	3.2	-0.5	2.9	-1.6	-1.6	1.1	-4.1
1	B	-2.4	-1.5	0.4	1.0	1.0	-4.2	1.1	-3.4	-1.5	-2.0	1.5	2.0
1	A	-0.4	1.0	2.4	-1.5	-1.0	-4.2	-1.1	-3.4	-1.5	2.0	1.5	-2.0
1	B	0.0	-7.8	0.0	-0.3	0.0	0.0	0.0	-0.1	0.0	0.0	0.0	0.0
1	A	0.0	-7.8	0.0	0.0	0.0	-0.3	0.0	0.0	0.0	0.0	0.0	-0.1

1 * Frame lines: 1 3

BRACING REACTIONS, PANEL SHEAR

Wall Loc	Col Line	± Reactions (k)				Panel Shear (lb/ft)
		Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	
L_EW	1	Rigid Frame At Endwall				
F_SW	A	1.2	1.6	1.2	7.8	6.0
R_EW	3	Rigid Frame At Endwall				
B_SW	B	2.1	1.6	1.2	7.8	6.0

- LOAD CASES**
- WIND_L1 = WIND LOAD FROM LEFT CASE 1
 - WIND_R1 = WIND LOAD FROM RIGHT CASE 1
 - WIND_LN1 = LONGITUDINAL WIND CASE 1
 - LWIND1_L2E = LONGITUDINAL WIND CASE 1 DUE TO EDGE ZONE 2E
 - SEISMIC_L = SEISMIC CASE FROM LEFT
 - SEISMIC_R = SEISMIC CASE FROM RIGHT
 - SEISMICLN = LONGITUDINAL SEISMIC CASE
 - F1UNB_SL_L = FRAME 1 UNBALANCED SNOW LEFT SIDE
 - F1UNB_SL_R = FRAME 1 UNBALANCED SNOW RIGHT SIDE
 - F1CRANE 1 = FRAME 1 CRANE LOAD IN POSITION 1
 - DRIFT = SNOW DRIFT LOAD
 - SLIDE = SLIDE SNOW LOAD

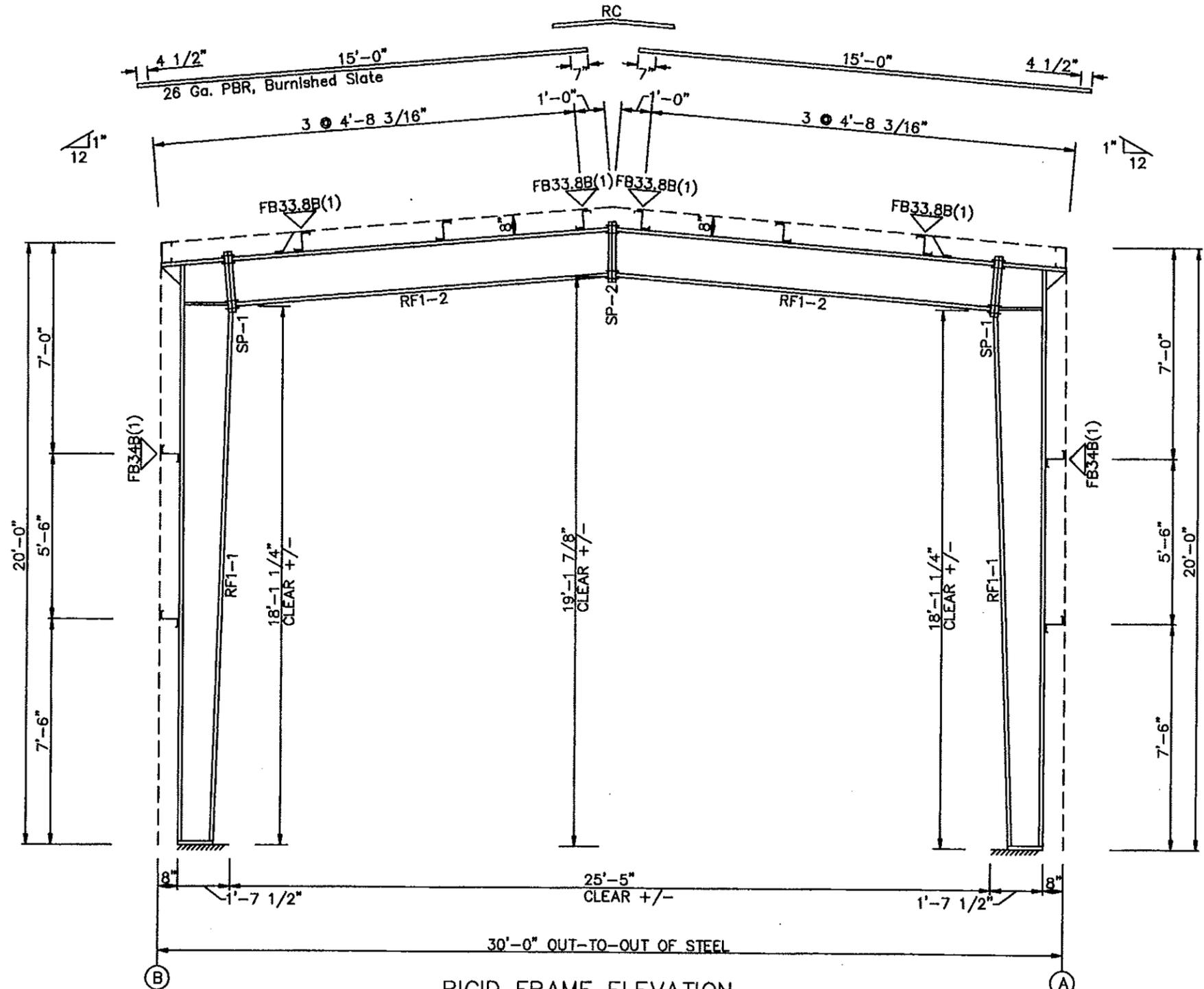


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City: VIRGINIA BEACH	State: VA	City: CHARLESTON	State: MO
Designer: SJ	Date: 12/22/10	Drafter: JMC	Date: 12/23/10
Detailer:	Date:	Office:	Job No.:
Checker:	Date:	Office:	15017
ANCHOR BOLT DETAILS & NOTES			Sht. E2 of 11

SPLICE BOLTS					
Splice Mark	Quan		Bolt		
	Top/Bot	Int	Type	Dia	Length
SP-1	4	4	0	A325T	0.750 2.50
SP-2	4	4	0	A325T	0.625 2.25

MARK	WEB DEPTH		WEB PLATE		OUTSIDE FLANGE		INSIDE FLANGE	
	START/END	THICK	LENGTH	W x T x LENGTH	W x T x LENGTH	W x T x LENGTH	W x T x LENGTH	
RF1-1	11.0/19.0	0.134	17'-9 1/16"	5" x 1/4" x 19'-3 7/8"	5" x 1/4" x 17'-9 3/16"			
	19.0/17.3	0.160	1'-8 5/16"	5" x 1/4" x 2'-1 5/8"				
RF1-2	16.5/16.5	0.134	12'-9 1/4"	5" x 1/4" x 12'-9 1/4"				

FLANGE BRACES: Both Sides(U.N.)
 FBxxB(1): xx=length(in), (1)=one side
 B - L2X2X10G (SEE DETAILS G1 & H1)



RIGID FRAME ELEVATION
 FOR FRAME LINE 2



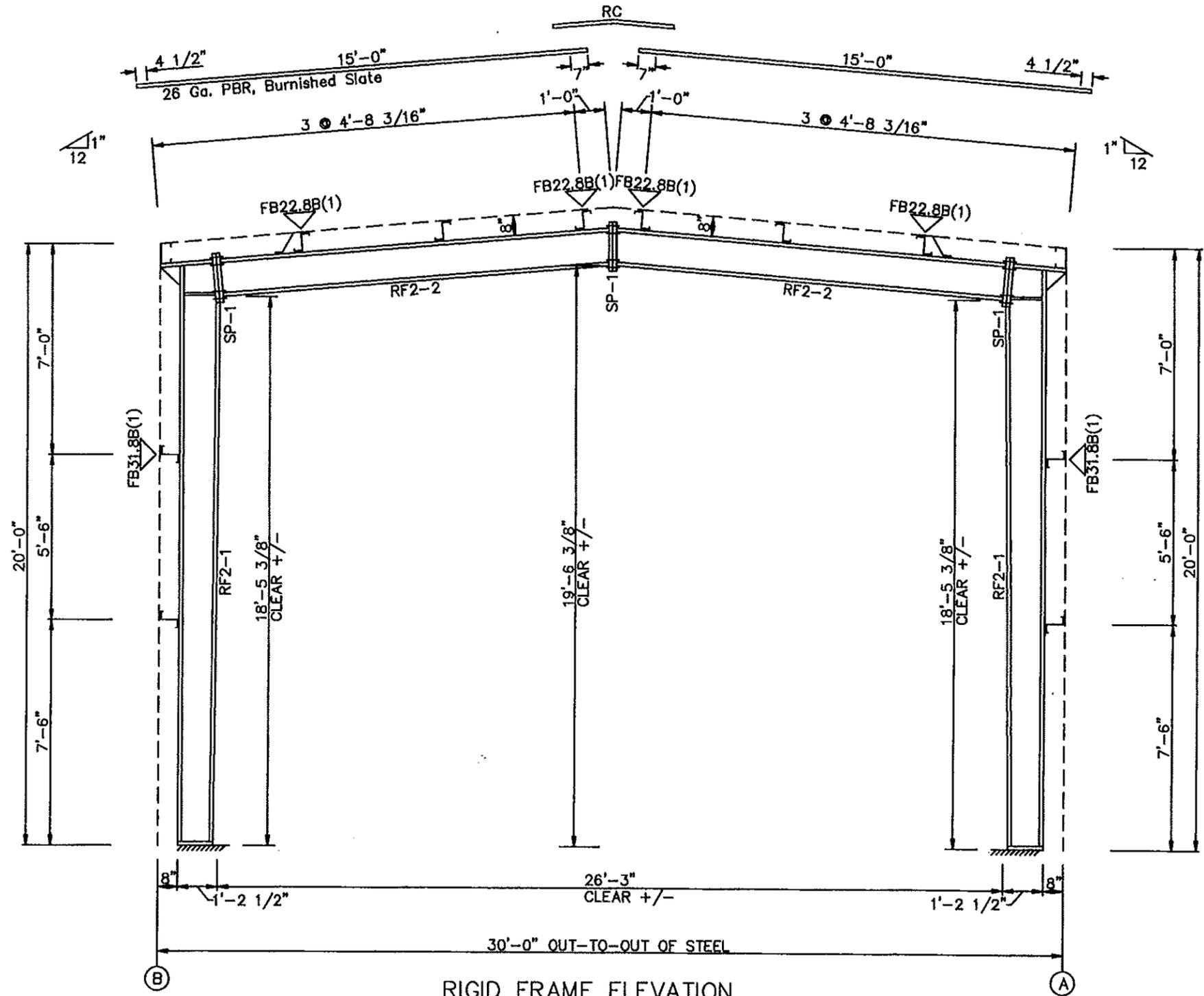
GENERAL NOTES:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

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RIGID FRAME ELEVATION			Sht. E3 of 11

SPLICE BOLTS					
Splice Mark	Quan	Top/Bot/Int	Type	Dia	Length
SP-1	4	4	0	A325T	0.625 2.25

MARK	WEB DEPTH		WEB PLATE		OUTSIDE FLANGE	INSIDE FLANGE
	START/END	THICK	LENGTH	LENGTH	W x T x LENGTH	W x T x LENGTH
RF2-1	11.0/14.0	0.134	19'-4 15/16"	5" x 1/4" x 19'-3 7/8"	5" x 1/4" x 18'-1 3/16"	
RF2-2	12.0/12.0	0.134	13'-1 7/8"	5" x 1/4" x 13'-1 7/8"	5" x 1/4" x 13'-0 7/8"	

✓ FLANGE BRACES: Both Sides(U.N.)
 FBxxB(1): xx=length(in), (1)=one side
 B - L2X2X10G (SEE DETAILS G1 & H1)



RIGID FRAME ELEVATION
FOR FRAME LINE 1 3



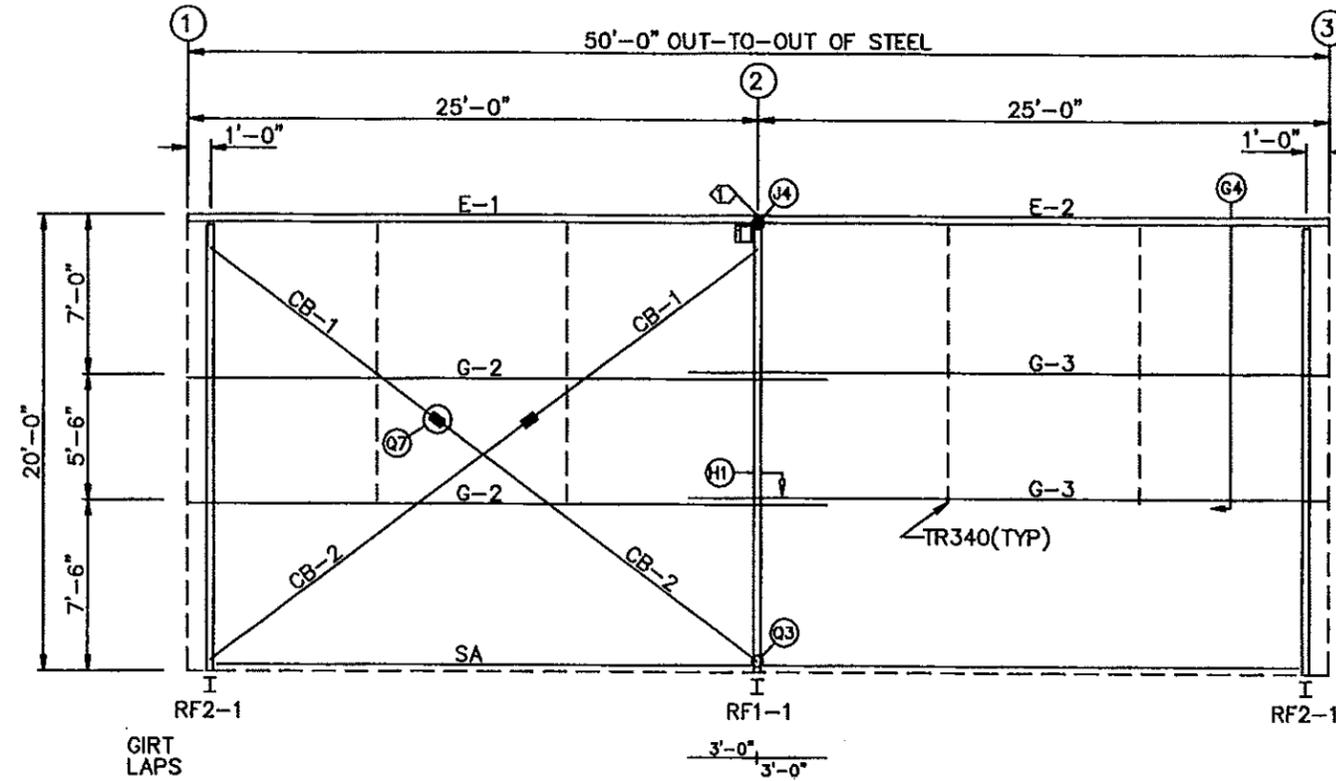
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RIGID FRAME ELEVATION			Sht. E4 of 11

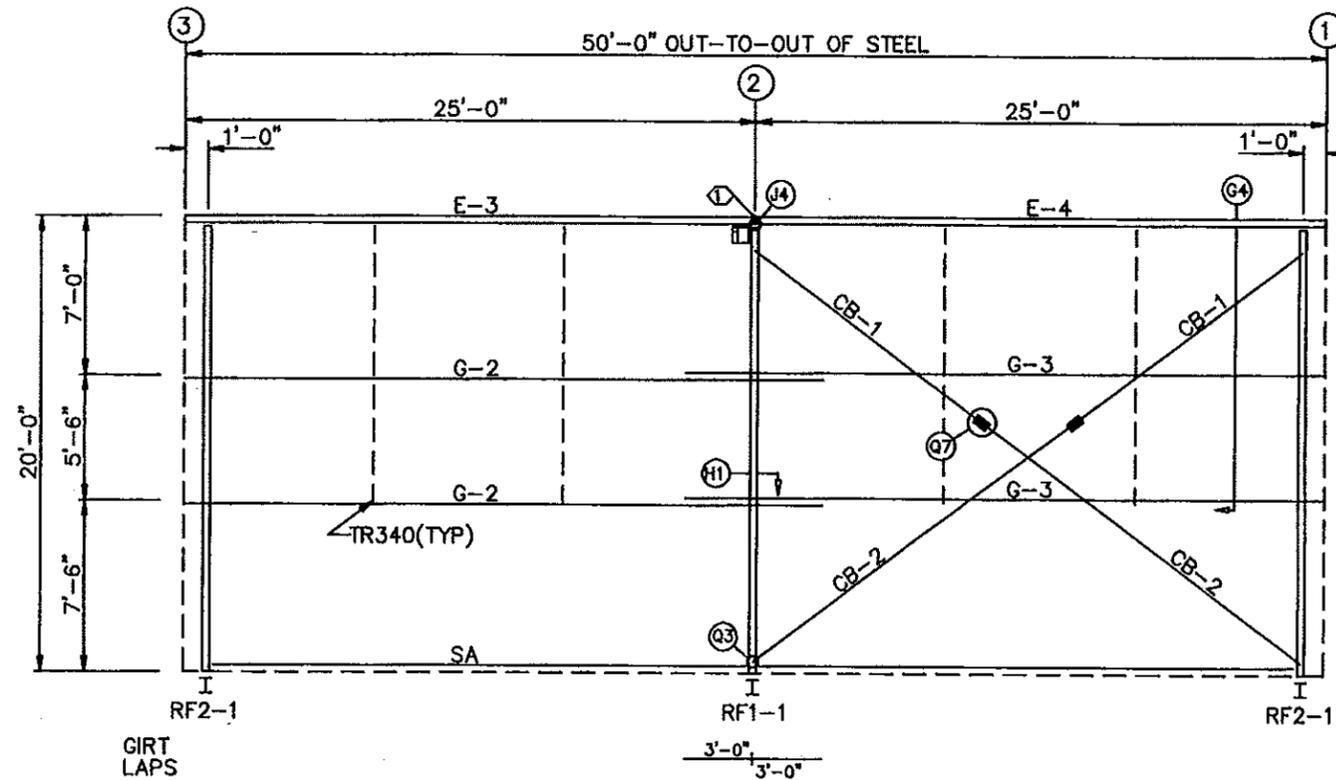
SPECIAL BOLTS					
Q ID	QUAN	TYPE	DIA	LENGTH	WASH
2	6	A325T	1/2"	2"	0

MEMBER TABLE FRAME LINE A & B		
MARK	PART	LENGTH
E-1	8x30E14	24'-11 1/4"
E-2	8x30E16	24'-11 1/4"
E-3	8x30E16	24'-11 1/4"
E-4	8x30E14	24'-11 1/4"
G-2	8x25Z15	27'-11 1/2"
G-3	8x25Z15	27'-11 1/2"
CB-1	0.75 ROD	10'-10"
CB-2	0.75 ROD	20'-0"

CONNECTION PLATES FRAME LINE A & B	
ID	MARK/PART
1	CL154
2	CLO50



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING: FRAME LINE B

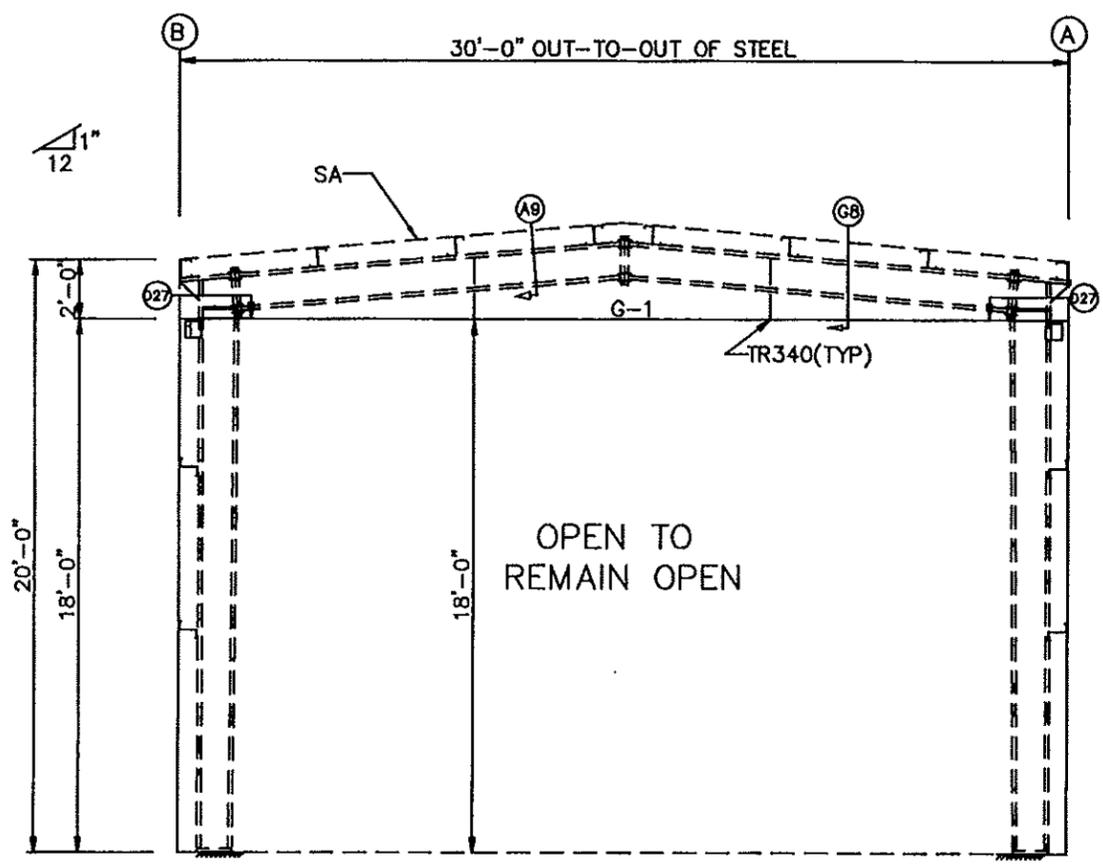


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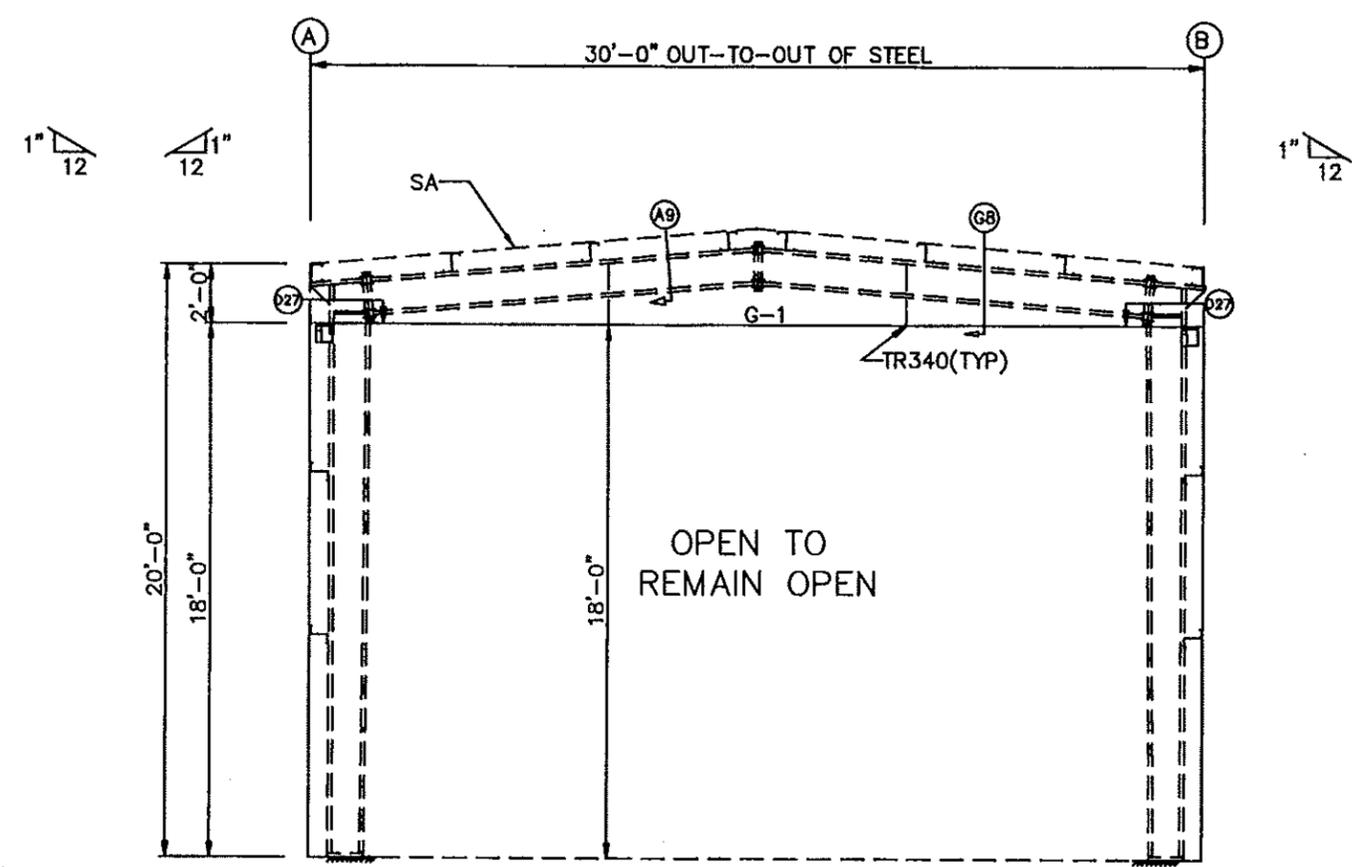
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Checker:	Date:	Office:	15017
SIDEWALL FRAMING			Sht. E5 of 11

MEMBER TABLE		
FRAME LINE 1 & 3		
MARK	PART	LENGTH
G-1	8x25C16	29'-11"

CONNECTION PLATES	
FRAME LINE 1 & 3	
ID	MARK/PART
1	CL034
2	CL156



ENDWALL FRAMING: FRAME LINE 1



ENDWALL FRAMING: FRAME LINE 3



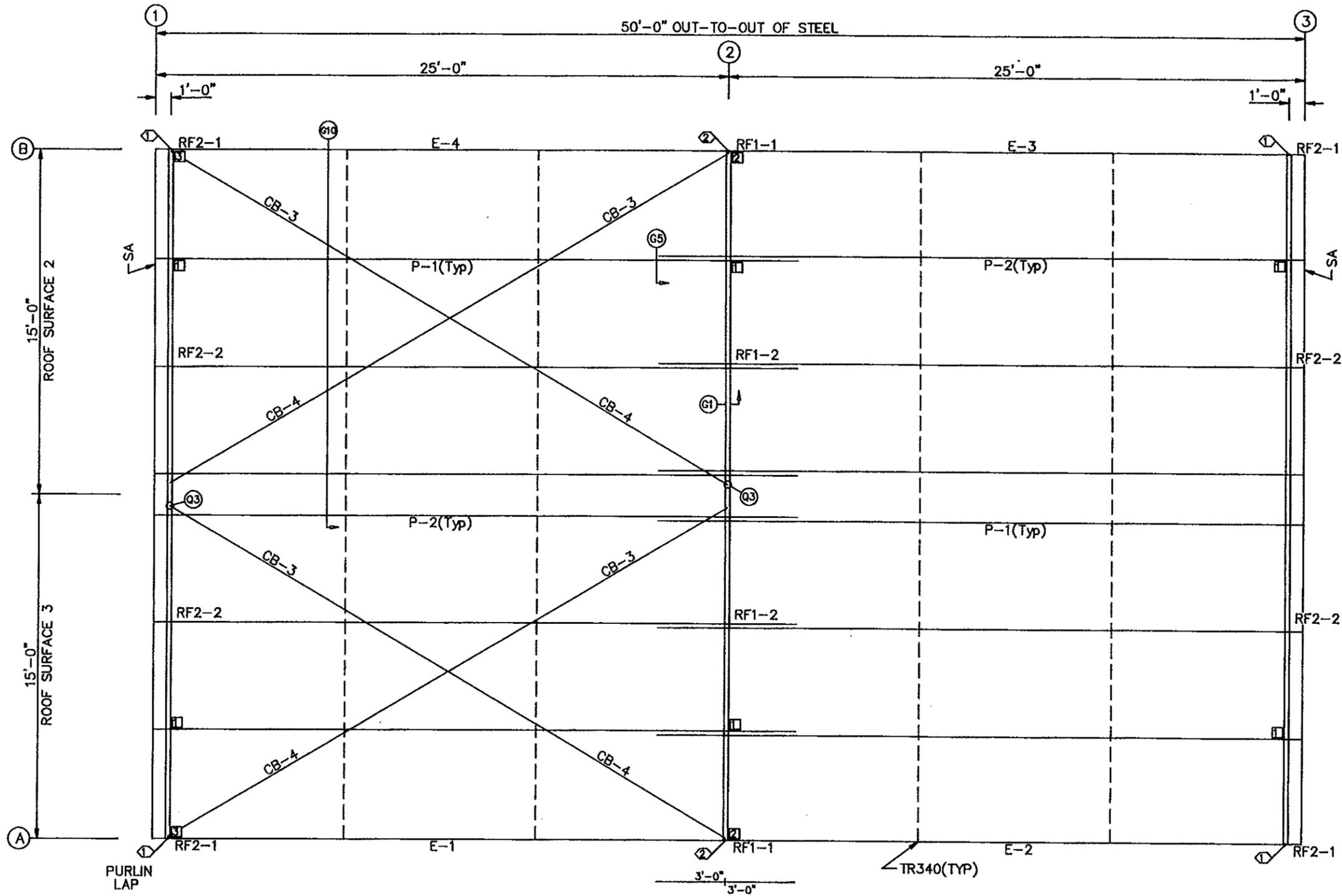
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ENDWALL FRAMING			Sht. E6 of 11

SPECIAL BOLTS					
ROOF PLAN					
Q ID	QUAN	TYPE	DIA	LENGTH	WASH
1	3	A325T	1/2"	2"	0
2	6	A325T	1/2"	2"	0

MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
P-1	8x25Z14	27'-11 1/2"
P-2	8x25Z14	27'-11 1/2"
E-1	8x30E14	24'-11 1/4"
E-2	8x30E16	24'-11 1/4"
E-3	8x30E16	24'-11 1/4"
E-4	8x30E14	24'-11 1/4"
CB-3	0.50_ROD	8'-0"
CB-4	0.50_ROD	20'-0"

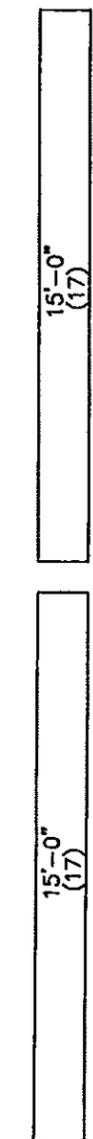
CONNECTION PLATES	
ROOF PLAN	
Q ID	MARK/PART
1	CL156
2	CL154
3	CL158



ROOF FRAMING PLAN

RC

 (17)



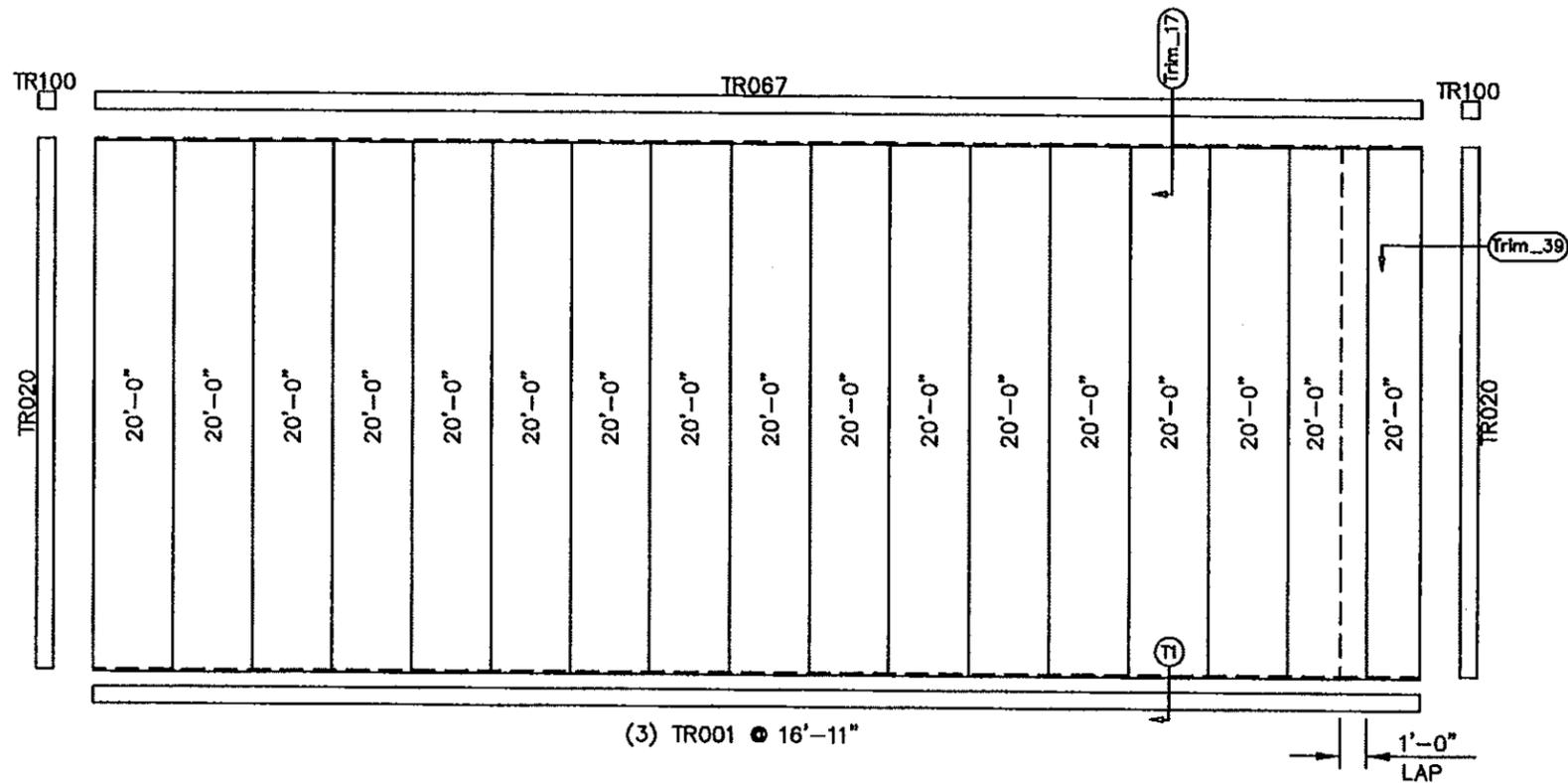
ROOF SHEETING
 PANELS: 26 Ga. PBR
 Burnished Slate



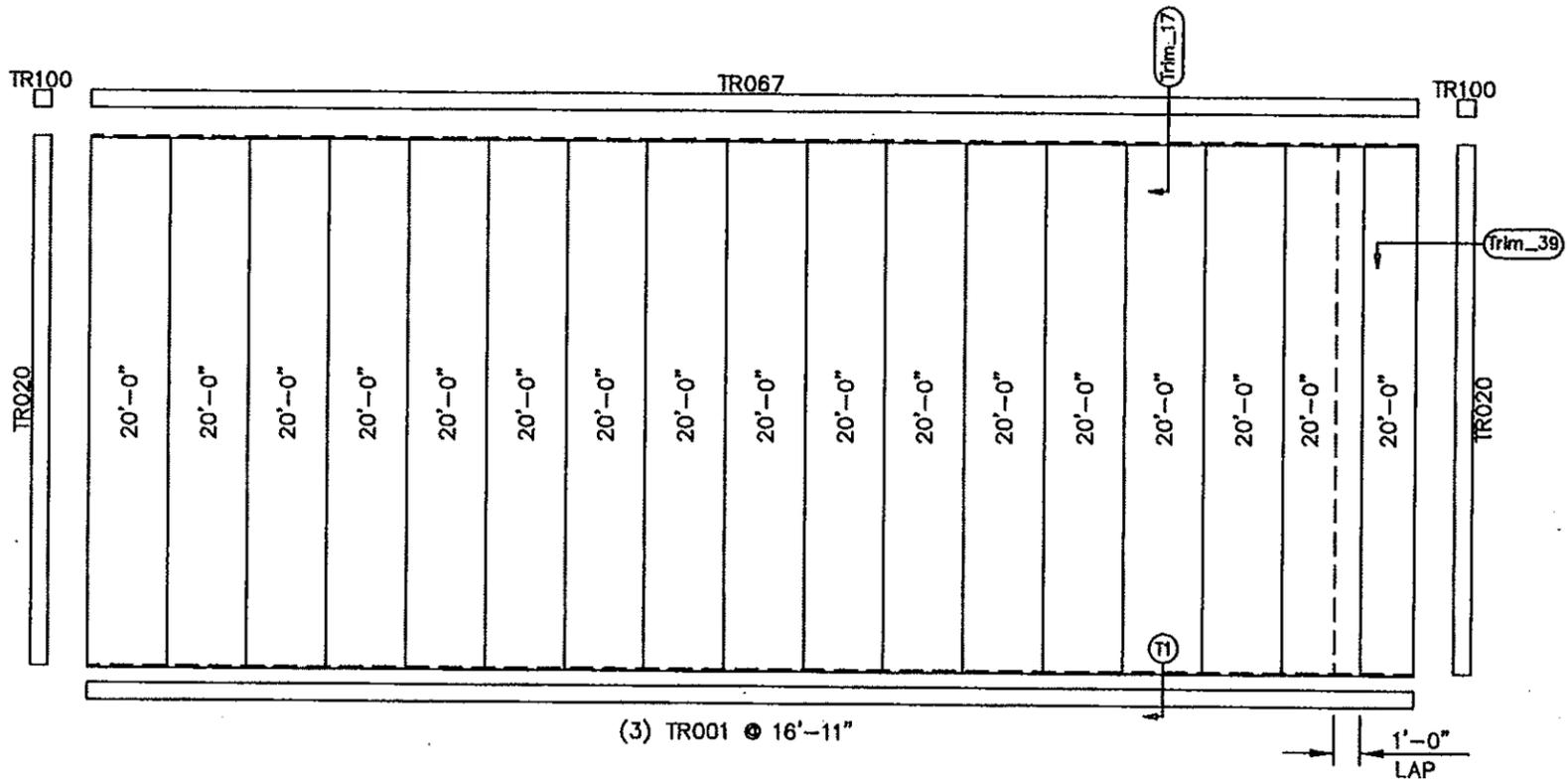
GENERAL NOTES:

MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. WE WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

POWERBILT STEEL BUILDINGS		Customer: MISSOURI D.O.T.	
City: VIRGINIA BEACH	State: VA	City: CHARLESTON	State: MO
Designer: SJ	Date: 12/22/10	Drafter: JMC	Date: 12/23/10
Detailer:	Date:	Office:	Job No.:
Checker:	Date:	Office:	15017
ROOF FRAMING & SHEETING			Sht. E7 of 11



SIDEWALL SHEETING & TRIM: FRAME LINE A
 PANELS: 26 Ga. GBR - Sahara Tan

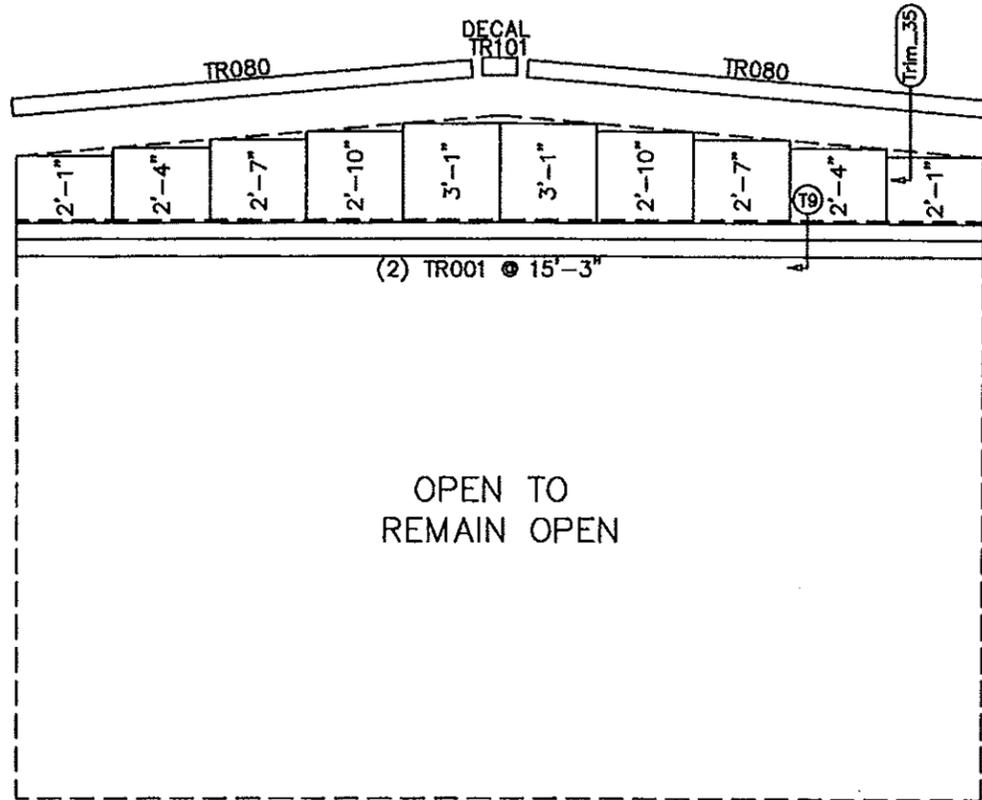


SIDEWALL SHEETING & TRIM: FRAME LINE B
 PANELS: 26 Ga. GBR - Sahara Tan

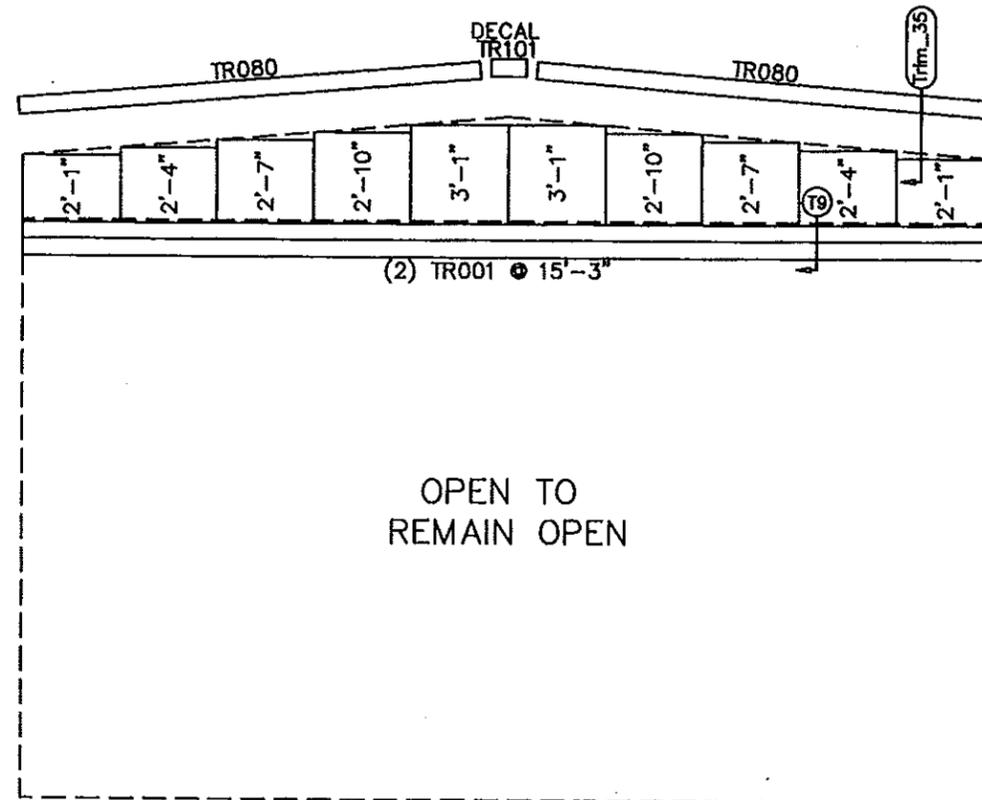


GENERAL NOTES:
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Detailer:	Date:	Office:	Job No.:
Checker:	Date:	Office:	15017
SIDEWALL SHEETING			Sht. EB of 11



ENDWALL SHEETING & TRIM: FRAME LINE 1
 PANELS: 26 Ga. GBR - Sahara Tan



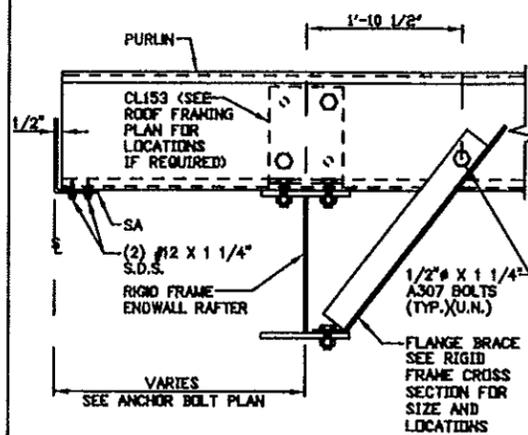
ENDWALL SHEETING & TRIM: FRAME LINE 3
 PANELS: 26 Ga. GBR - Sahara Tan



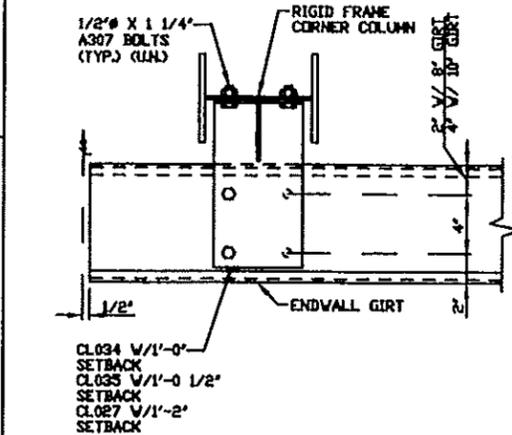
GENERAL NOTES:

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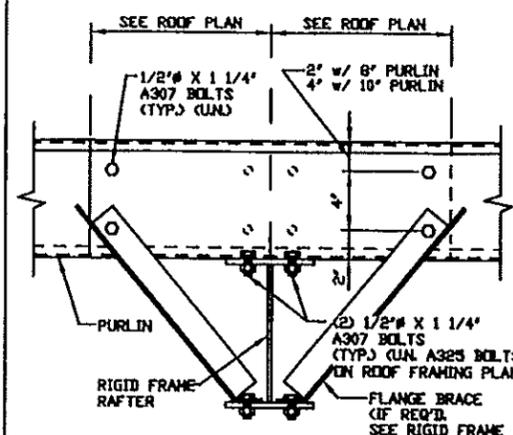
POWERBILT STEEL BUILDINGS		Customer: MISSOURI D.O.T.	
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Checker:	Date:	Office:	15017
ENDWALL SHEETING			Sht. E9 of 11



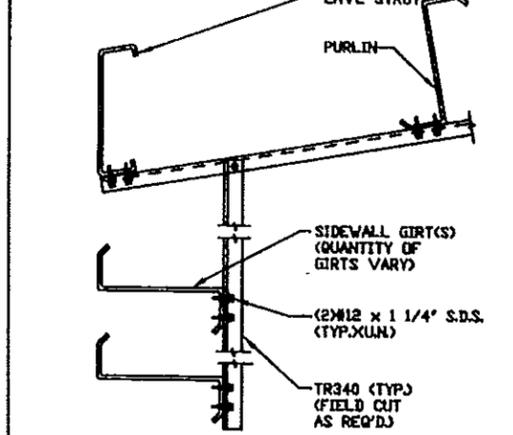
A9 R.F. ENDWALL RAFTER (OPEN WALL)



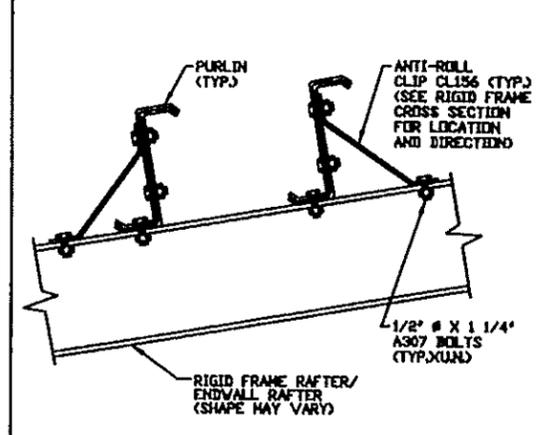
D27 CORNER COLUMN GIRT CONNECTION



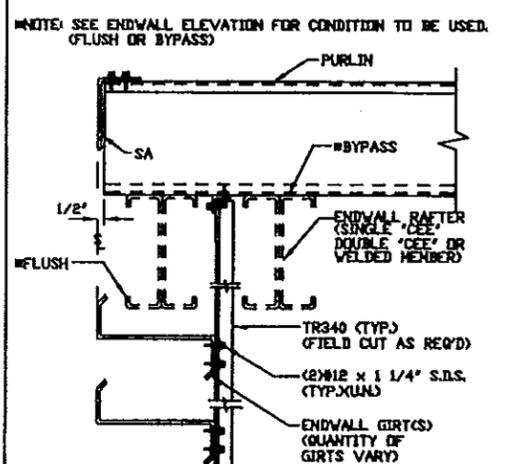
G1 FLANGE BRACE/BOLT PATTERN AT PURLIN LAP



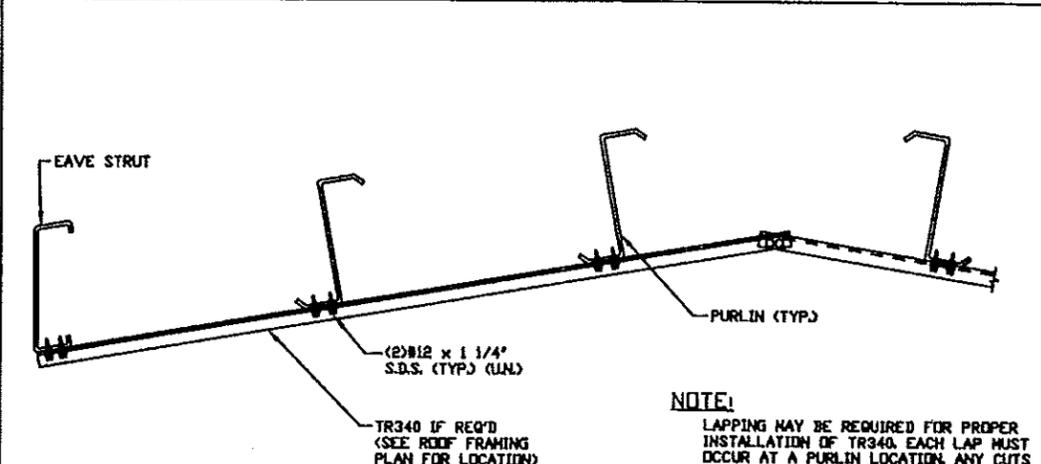
G4 SIDEWALL GIRT SAG ANGLE



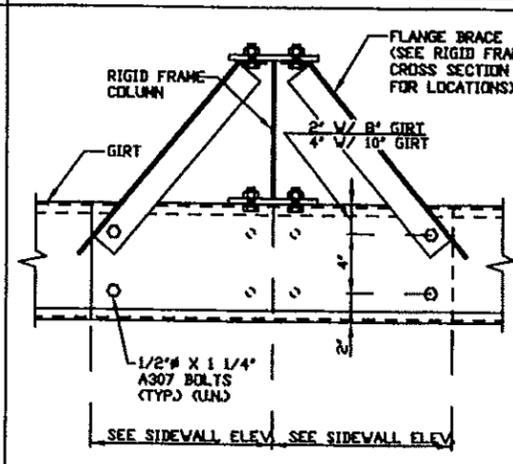
G5 ANTI-ROLL CLIP DETAIL



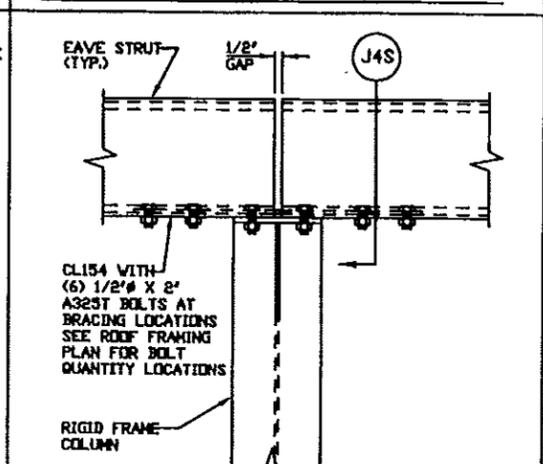
G8 ENDWALL GIRT SAG ANGLE



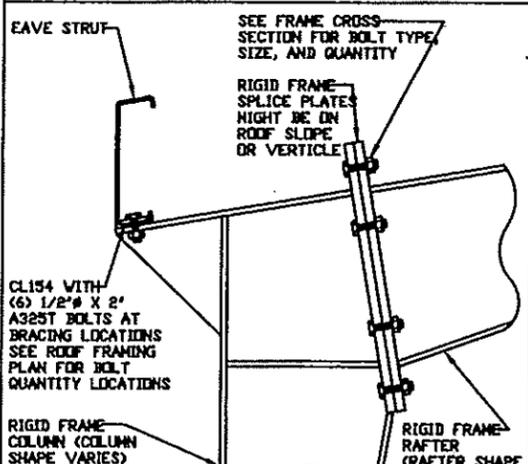
G10 PURLIN SAG ANGLE DETAIL



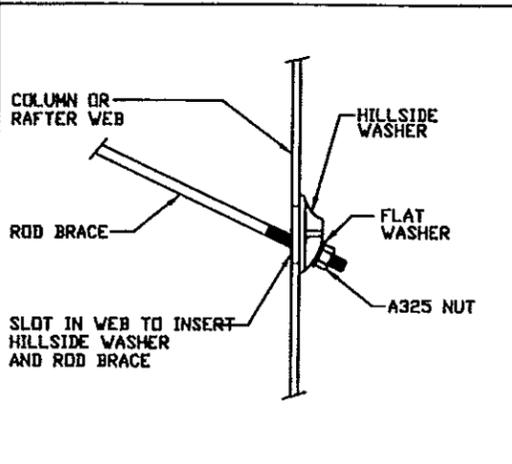
H1 2x2 SIDEWALL GIRT CONNECTION



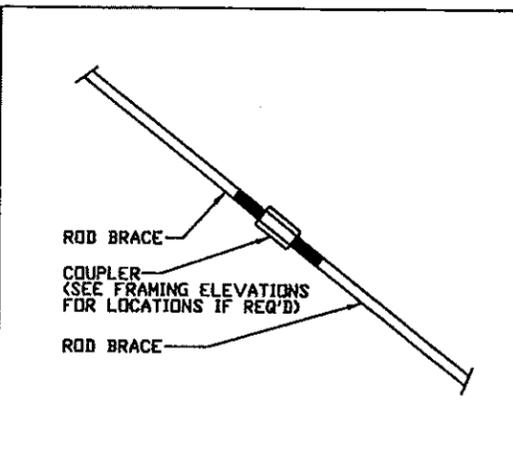
J4 EAVE STRUT TO RIGID FRAME



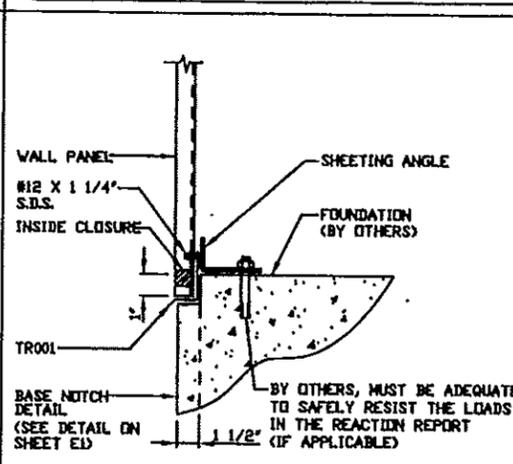
J4S EAVE STRUT TO RIGID FRAME



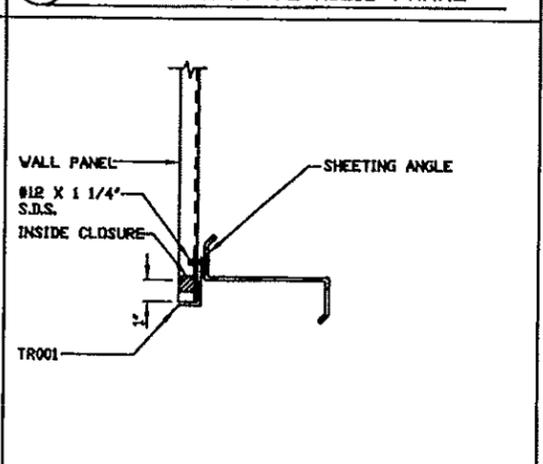
Q3 DIAGONAL ROD BRACE CONNECTION



Q7 INTERMEDIATE ROD BRACE SPLICE



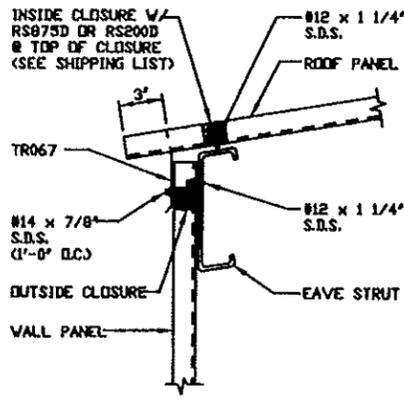
T1 BASE TRIM SECTION



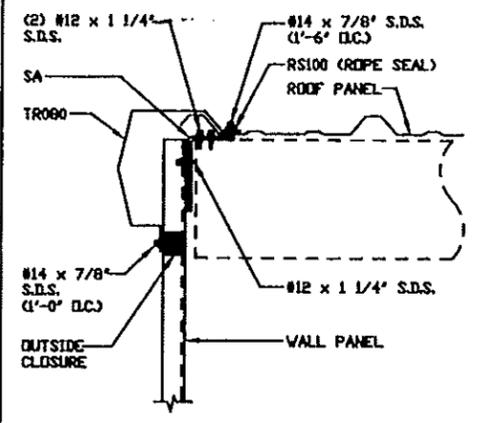
T9 BASE TRIM SECTION

NOTE: ALL SCREWS ARE WITH WASHERS UNLESS NOTED

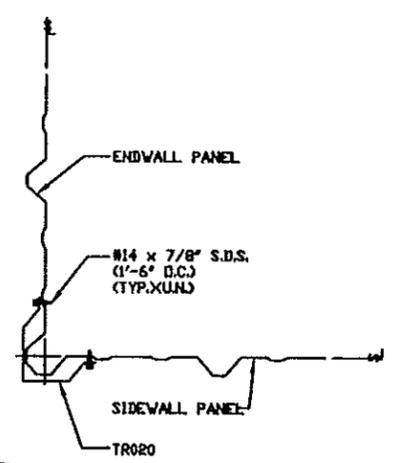
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DETAIL DRAWINGS			Sht. E10 of 11



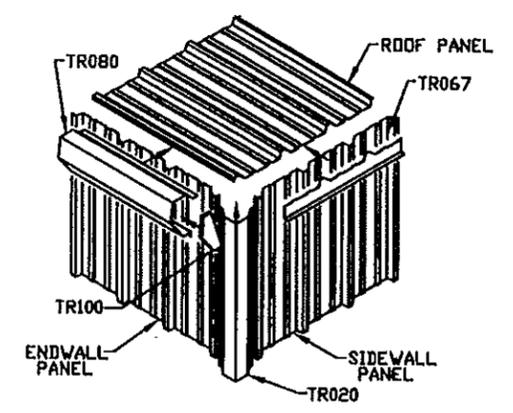
Trim_17 EAVE TRIM SECTION



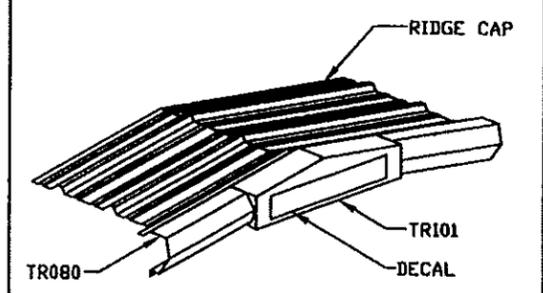
Trim_35 RAKE TRIM SECTION



Trim_39 CORNER TRIM SECTION



BUILDING CORNER DETAIL



PEAK DETAIL

POWERBILT STEEL BUILDINGS		Customer: MISSOURI D.O.T.	
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Checker:	Date:	Office:	
TRIM DRAWINGS			Sht. E11 of 11